COMMENTS ABOUT THE BENEFITS OF FINITE ELEMENT MODELING FOR: SOIL STRUCTURE INTERACTION ANALYSIS OF DEEP FOUNDATIONS

Dr. Eng. Özgür BEZGİN

İSTANBUL January 2010

CONCLUSIONS

A three dimensional finite element continuum model was developed for the lateral response of drilled shafts. The developed model included most of the parameters that influence the lateral response of shafts. The model was used to analyze single shafts, group shafts, and the results were compared to less refined models and experimental results.

Based on the results obtained in this dissertation, the following conclusions can be made:

1.1 Finite Element Optimization

- The effective volume of soil that needs to be included in the finite element model depends on the exclusion or inclusion of soil selfweight deformations within the lateral load analysis. For the models that do not include the soil selfweight, the volume of soil that needs to be included within the model should not be less than 2.5D from the shaft surface. For the models where the soil selfweight are included in the analysis, the volume of the soil that needs to be included within the model depends on the thickness of the soil layer and the angle of friction of the soil.
- Use of four solid elements per quadrant is sufficient to define the radial spread of the shaft-soil cross section. The use of finite elements with the size equal to 2.5 percent of the depth of the shaft is sufficient define the longitudinal meshing for slender shafts with depth to diameter ratios higher than 8. For ratios lower than 8, finite element with sizes 5 percent of the shaft depth is sufficient. The division of the lateral extension of the soil by finite elements with sizes 10-15 percent of the amount of soil extension from shaft surface is sufficient.

The use of reduced integration linear elements for the shaft, full integration linear elements for the soil, and infinite elements for the boundary soil region gave good results and is recommended for three-dimensional FE analysis. Quadratic elements tend to cause convergence problems and instability and therefore are not recommended when modeling soil-shaft surface interaction. The use of infinite elements for soil at model boundaries rather than boundary conditions is an efficient technique to model soil continuity.

1.2 Lateral Response of Drilled Shaft

- Neglecting surface friction at the interface overestimates shaft displacements and moments. The effect of friction on shaft response was also dependent on the depth to diameter ratio of the shaft and support conditions at the bottom of the shaft.
- For shafts with depth to diameter ratios of 8 or higher, the average reduction in displacement due to an interface static friction constant of 0.2 is 3 percent, for 0.5 is 6 percent, and for 0.9 is 8 percent. The average reduction in moment due to an interface friction coefficient of 0.2 is 1.5 percent, for 0.5 is 3 percent, and for 0.9 is 4.5 percent. The shaft support conditions influence the effect of interface friction for shafts with depth to diameter ratios below 8. For shafts fixed at the bottom, the effect of friction on the shaft response due to lateral loads was less pronounced compared to that of shafts normally supported by rigid surface.
- Including the effects of Poisson's ratio in the analysis increased the shaft response to lateral loading. The inclusion of Poisson's ratio of 0.2 has increased the maximum shaft displacement by 7 percent, and the maximum moment by 1 percent. The inclusion of Poisson's ratio of 0.4 increased the

maximum shaft displacement by 11 percent and the maximum moment by 2 percent.

- Including soil weight in the analysis increases the stiffness of the shaft-soil system. The maximum shaft displacement was reduced by 25 percent and the maximum moment was reduced by 6 percent. The analysis showed that for every 10pcf of increase in unit weight of cohesionless soils, the displacement of the shaft was reduced by about 2 percent and the maximum moment by about 0.5 percent.
- The conditions of the soil in close proximity to the shaft surface had a major impact on the shaft response. As a result of the analysis it was found that the amount of the extension of weak soil zone around the shaft as well as the amount of strength reduction within this zone highly influences the lateral capacity of the shaft, which has to be taken into account in design. This phenomenon can be analyzed efficiently with the FE modeling.
- Depth of soil up to 5 shaft diameters below the ground surface is the most effective depth of soil in determining the lateral response and load capacity of a drilled shaft. In addition, this zone is more susceptible to surface separation due to cyclic loads making it a critical parameter. Loss of soil support in this region had a major impact on shaft capacity for slender shafts (Z > 5). Loss of soil support that extends 2.5 shaft diameters below the ground surface increases the maximum shaft displacement by 73 percent and the maximum moment by 42 percent.
- Shaft support conditions at the bottom start to influence the shaft response for shafts with depth to diameter ratios below 10 (Z < 5). For depth to diameter ratio of 10, the ratio of the moment developed at the support to maximum moment was about 13 percent. This percentage increases as the

shaft depth decreases. For shaft depth to diameter ratio of 8, the maximum displacement and the maximum moment vary by 15 percent and 5 percent for fixed support and normal support respectively. The support condition at the bottom of shaft needs to be accurately modeled.

The approximate length of fixity predicted by the developed FE model was about 15 percent to 20 percent lower than the length of fixity estimated by less refined methods used in design practices. This indicates that the less refined methods overestimate the shaft maximum moments and maximum displacements.

1.3 Group Shafts

- As expected, the center-to-center spacing of the shafts influences the stiffness of the shaft group. As the spacing is decreased, the lateral support from the soil is reduced, thus increasing shaft displacements, and moments.
- Group coefficients were obtained for the effect of shaft spacing on the maximum shaft displacements. These coefficients are defined as the ratio of the group displacement to that of a single shaft. Group shaft analysis showed that, for practical shaft spacing (3D to 4D), the increase in groupdisplacement was about 50 to 60 percent.
- Support moments are observed for shafts in close proximity (for center-tocenter spacing of 3D).

1.4 Comparisons with Existing Models

Finite element modeling with soil continuum predicted lower displacement and moments compared to the spring model and the LPILE.

- Comparison of the Type-1 FE model and the spring model confirmed the effect of shear coupling on lateral soil resistance. It was found that including shear coupling as an addition to individual spring stiffness decreased the displacements by 5 percent and the moments by 3 percent.
- Comparison of Type-2 FE model with the Type-1 FE model has shown the effect of soil confinement on the stiffness of the shaft. The FE model showed that including the soil weight reduced the displacements by about 25 percent and reduced the moments by 5 percent.
- Effectiveness of the shaft depth in resisting lateral loads depends on the relative stiffness of the shaft and the soil, as well as the depth of the shaft. The maximum depth coefficient Z_{max} beyond which the extension of the shaft has no effect on lateral loads resistance is found to be: Z_{max} =5. This conclusion confirms Matlock and Reese (1962) criteria for long shafts beyond which extension in depth has no effect on lateral load resistance of the shaft.

1.5 Suggestions for Further Research

- FE modeling is an effective approach to deep-foundation modeling.
 However, further field verification is needed to compare and calibrate the FE model in order to generate design-based guidelines.
- There is a need to establish design charts for lateral load design of drilled shafts. To this end, further analytical studies regarding the influence of the stiffness parameters of soil and shaft to lateral response of the shaft are needed.
- > There is a need to study multi-layered soil profiles and cohesive soils.

- There is a need to study the shaft and soil response under the action of dynamic and other time dependent loads.
- Soil strength reduction associated with drilling operations is shown to have an important effect on lateral response of the shaft. Further in-depth investigation is needed to fully evaluate the associated effects on design capacity of a shaft under lateral loads.

REFERENCES

- 1. ABAQUS Standard/Explicit Manuals versions 6.3&6.4
- 2. Anderson, Townsend. (2002) "A Laterally Loaded Pile Database". Deep Foundations 2002: An International Perspective on Theory, Design, Construction, and Performance pp. 262-273
- Baker (1993). "Use of Pressuremeter in Mixed Highrise Foundation Design". Design and Performance of Deep Foundations: Piles and Piers in Soil and Soft Rock pp. 1-13
- 4. Boulanger, Hutchinson, Chai, Idriss (2004). "Estimating Inelastic Displacements for Design: Extended Pile-Shaft-Supported Bridge Structures "Earthquake Spectra, Vol. 20, No. 4, pp. 1081-1094.
- 5. Bowles (1997). "Foundation Analysis and Design 5th Edition" MacGraw-Hill Companies
- 6. Briaud, Buchanan (2000) "Introduction to Soil Moduli".
- 7. Briaud, L.L., Smith, T.D. Tucker, L. (1985). "Pressuremeter Design Method for Laterally Loaded Piles," *Proceedings of the XI International Conference on Soil Mechanics and Foundation Engineering*, San Francisco, CA, U.S.A.,
- 8. Briaud, Johnson, Stroman (1984). "Lateral Load Test of an Aged Drilled Shaft" Laterally loaded deep foundations: Analysis and performance, STP 835 pp. 172-181
- 9. Britto, Gunn, (1987) *Critical State Soil Mechanics Via Finite Elements* Ellis Horwood Limited
- 10. Chen, Liu (1990). "Development in Geotechnical Engineering 52 -Limit Analysis and soil plasticity" -Elsevier Publishing
- 11. Chen, Mizuno (1990) "Developments in geotechnical engineering53 -Nonlinear Analysis in Soil Mechanics Theory and Implementation"- Elsevier Publishing
- 12. Chen (1975) "Development in Geotechnical Engineering 7 -Limit Analysis and soil plasticity"-Elsevier Publishing
- 13. Choi, Oh, Kwon, Kim. (2002) "A Numerical Analysis for Axial and Lateral Behavior of Instrumented Steel Pipe Piles". Deep Foundations 2002: An International Perspective on Theory, Design, Construction, and Performance pp. 289-304
- 14. Cook, Malkus, Plesha, Witt (2002). "Concepts and Applications of Finite Element Analysis 4th Edition" John Wiley and Sons Inc.

- 15. Dameron, Arzoumanidis, Bennett, Malik (1999). "Seismic Analysis and Displacement Based Evaluation of the Brooklyn-Queens Expressway".
- 16. Das (1999) "Principles of Foundation Engineering 4th edition" PWS Publishing
- 17. Dessai, Abel (2002). "Introduction to Finite Element Modeling". CBS Publishers & Distributers
- 18. Duggal, Bohinsky, Chu. (1989) "Comparative Performance of Two Pile Types" Foundation Engineering, pp. 943-956
- 19. Habigaghi, K. and Langer, J.A. (1984). "Horizontal Subgrade Modulus of Granular Soils". Laterally loaded deep foundations: Analysis and performance, STP 835 pp. 21-34
- 20. Horvath, (1984). "Simplified Elastic Continuum Applied to the Laterally Loaded Pile Problem". Laterally loaded deep foundations: Analysis and performance, Laterally loaded deep foundations: Analysis and performance, STP 835 pp. 229-238
- 21. Horvath J.S. (2002) "Soil-Structure Interaction Research Project:Basic SSI Concepts and Applications Overview" Report No. CGT-2002-2
- 22. Huang, Ye, Tang. (2002) "Dynamic Coupled Analysis for Earthquake Response of Pile Foundations". Deep Foundations 2002: An International Perspective on Theory, Design, Construction, and Performance pp. 396-404
- 23. Kappos, Sextos (1999) "Effect of Foundation Type and Compliance on Seismic Response on RC Bridges". Journal of bridge engineering, Vol.6, No.2, March 2001.
- 24. Kulhawy. (2002) "Observations on Some Shortcomings in Foundation Analysis and Design". Deep Foundations 2002 (GSP 116), pp.1-5.
- 25. Kulhawy, Cushing.(2002) "Drained Elastic Behavior of Drilled Shafts in Cohesionless Soils". Deep Foundations 2002: An International Perspective on Theory, Design, Construction pp. 22-36
- 26. Kulhawy, Agaiby, Trautmann (1996) "On large scale model testing of laterally loaded drilled shafts in sand " Geotechnical Testing Journal , vol.v19., no.n1., pp.pp32-40.
- 27. Kulhawy, F. H. (1991). "Drilled shaft foundations , Foundation engineering handbook".
- 28. Kumar, Kort, Hosin, and Chong (2004) "Lateral Load Tests on Small Diameter Drilled Piers"
- 29. <u>Kumar,</u> Alizadeh (2002). "Lateral Load-Deflection Response of Single Piles in Sand".

- 30. <u>Kort, Kumar, Hosin, Ng (2002)</u> "Lateral Load Tests on Small Diameter Drilled Piers".
- 31. Lin, Yang, Juang, Lee (2000). "Analysis of Laterally Loaded Piles in a Two-Layered Elastic Medium".
- 32. Lee, Kane, Bennett, Drumm (1989) "Investigation and Modeling of Soil-Structure Interface Properties"
- 33. Lee (1991) "Discrete Layer Analysis of Laterally Loaded Piles".
- 34. Long, Reese (1984). "Testing and Analysis of Two Offshore Drilled Shafts Subjected to Lateral Loads". Laterally loaded deep foundations: Analysis and performance, STP 835 pp. 215-228
- 35. Luna, Jadi (1998) "Determination of Dynamic Soil Properties Using Geophysical Methods".
- 36. Macklin, Nelson, Chou (1993) "A Lateral Load Test on Seven Foot Diameter Caissons".
- 37. <u>Maharaj (1997)</u>["]Load-Deflection Response of Laterally LoadedSingle Pile by Nonlinear Finite Element Analysis".
- 38. Matlock, Reese, (1960). Generalized Solutions for laterally Loaded Piles, Journal of the Soil Mechanics and Foundations Division, ASCE, Vol.86, No SM5, Proc.Paper 2626, pp.63-91
- 39. Motan, Gabr. (1989) "A Flat-Dilatometer Study of Lateral Soil Response."
- 40. Neate (1983) "Augered Cast in Place Piles".
- 41. Neely (1979) "Bearing pressure-SPT Correlations for Expanded Base Piles in Sand".
- 42. Olson, Clifford, Wright (1983) "Nondestructive Testing of Deep Foundation with Sonic Methods".
- 43. Petek, Felice, Holtz. "Capacity Analysis of Drilled Shafts with Defects".
- 44. Pise, P. J. (1983), Lateral Response of Free-Head Pile, Journal of Geotechnical Engineering, ASCE, Vol. 109, No.8 pp. 1126-1131.
- 45. Popov (1998) "Engineering Mechanics of Solids 2nd Edition" Prentice Hall Publishing.
- 46. Prakash, Sharma (1990) "Pile Foundations in Engineering Practice". John Wiley and Sons Inc.
- 47. Puppala, Moalim (1986) "Evaluation of Driven Pile Load Capacity Using CPT Based LCPC and European Interpretation Methods".

- 48. Pyle, R. and Beikae, M. (1984). "A New Solution for the Resistance of Single Piles to Lateral Loading" Laterally Loaded Deep Foundations: Analysis and Performance, STP 835 835 pp. 3-20
- 49. Reese, Wright, Aurora (1984). "Analysis of a Pile Group Under Lateral Loading". Laterally loaded deep foundations: Analysis and performance, STP 835 pp. 56-71
- 50. Reese, L.C., and Matlock, H., (1956). "Non-Dimensional Solutions for Laterally Loaded Piles with Soil Modulus Assumed Proportional to Depth", Proceedings, Eighth Texas Conference on Soil Mechanics and Foundation Engineering,
- 51. Roberston, Hughes (1984). "Design of Laterally Loaded Displacement Piles Using a Driven Pressuremeter". Laterally loaded deep foundations: Analysis and performance, STP 835 pp. 229-238
- 52. Sogge (1984). "Microcomputer Analysis of Laterally Loaded Piles". Laterally Loaded Deep Foundations: Analysis and Performance, STP 835 pp. 35-48.
- 53. Smith, T.D., (1989) "Fact or Friction: A Review of Soil Response to a Laterally Moving Pile", *Proceedings of the Foundation Engineering Congress*, Northwestern University, Evanston, Illinois, pp. 588-598
- 54. Smith, T.D., Slyh, R. (1986) "Side Friction Mobilization Rates for Laterally Loaded Piles from the Pressuremeter, "Proceedings of the Second International Symposium, The Pressuremeter and its Marine Application", Texas A&M, May ASTM STP 950, pp. 478-491
- 55. Taciroglu, Rha, Stewart, Wallace, (1999). "Robust Numerical Models for Cyclic Response of Columns Embedded in Soil".
- 56. Vennalaganti, Endley, Rao (1992) "Lateral Loads on Long piles and piers in granular soils".
- 57. Wang, Rinne (1999) "Pile Foundation Construction Practice in Stiff Clays with Dense Granular Layers".
- 58. Woodward, Gardner, Greer. (1972) "Drilled Pier Foundations" McGraw Hill Publishing
- 59. Zhang, Tulla, Grismala (1977) "Ultimate Resistance of Laterally Loaded Piles in Cohesionless Soils".
- 60. Zafir (1986) "Seismic Foundation Stiffness for Bridges".